



Seismic microzoning in Skopje, Macedonia



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ABSTRACT

Seismic microzonation maps for Skopje (Macedonia) are presented based on the Uniform-Hazard-Spectrum (UHS) methodology. UHS satisfies the guidelines for performance-based design (PBD), which require specification of two sets of spectral amplitudes, one for which the structure will remain essentially linear, and the other for which it will undergo a nonlinear response. The UHS method also considers the contributions from large distant earthquakes and includes simultaneous effects of site geology and site soils. The maps presented include the effects of near and distant large earthquakes in a balanced way, spatial distribution of seismic activity, site geology, and site soil properties.

1. Introduction

The design of structures to withstand earthquake shaking continues to be based on the expected amplitudes of strong earthquake ground motion, which are characterized in terms of maxima in the dynamic response, as given by the response-spectrum method [15,72,81]. Investigations and observations of damage caused by earthquakes, have shown that the observed variations are related to the geologic properties and soil site conditions. To account for such variations, microzoning maps can be formulated with coefficients that describe the variations in the amplitudes of shaking, and hence in the design forces [26,50,58]. The equivalent horizontal earthquake force, or the response-spectrum amplitudes for design, are then increased or decreased according to the values of the amplification coefficients defined in the microzoning maps.

The early development of seismic microzoning maps dates back to the Soviet Union (Akademia [1]) and Japan in the 1930s [26]. On the basis of many observations following earthquakes, guidelines were developed for the prediction of the relative increase or decrease of site intensities (and then of the associated peaks of strong motion amplitudes) based on the nature of site geology and surface soil [14,50]. Many published seismic microzonation maps from that time resembled the spatial distribution of geological and soil deposits in the area [27,48,50,58]. The local spatial variations were first based primarily on site geology [50,58] and later expanded to include the effects of

shallow sediments and local soils. Theoretical and observational studies in Japan [24,25] later evolved into methods that also included the properties of local site characteristics determined through field measurement of microtremors. After many years of research and the eventual introduction of probabilistic methods for evaluation of seismic design forces, we are now re-discovering that the old methods correctly recognized and emphasized the importance of both the site soil and site geology in influencing the spatial variations in seismic hazard. The results in this paper will show that site geology is indeed important for mapping seismic hazard and therefore must be included in the empirical scaling equations of strong ground motion.

Since the mid-1970s, after the first direct empirical scaling equations of spectral amplitudes started to appear, it became possible to formulate seismic zoning and microzoning in ways that considered the probabilities of earthquake occurrence, the spatial distributions of earthquake sources, the frequency-dependent attenuation of strong-motion amplitudes, and the site geologic and soil conditions [30–32,34,35,42–45,62–69,71,76]. The advantage of this approach has been that it simultaneously considers the contributions of all factors that influence the end result. Comparisons with earthquake occurrence in southern California have confirmed the merits of this approach. For example, seismic microzonation maps based on the uniform hazard method (UHM) [2,3] that were calculated and published in 1987 [36] for the Los Angeles metropolitan area have not been contradicted by any strong motion shaking from the earthquakes that have occurred in

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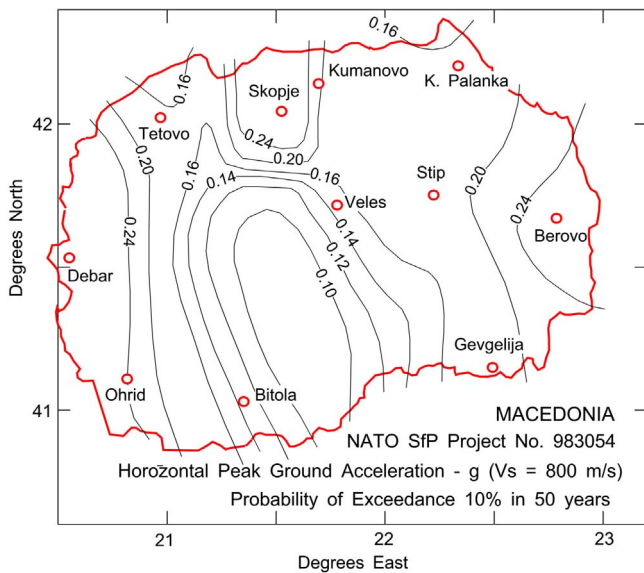


Fig. 1. A preliminary map of peak accelerations (g), at site A (800 m/s), for a probability of 10% exceedance, and an exposure period of = 50 years (redrawn from [59]).

the area since 1985 [70,74].

The purpose of this paper is to show how a model of seismic activity in the region, as well as the inclusion of geological site characterization (as in [77]), can be used to formulate microzoning maps of Skopje, Macedonia. The methodology, scaling equations, and descriptions of seismicity used in this paper are same as those described by [46,47,40–42] and will not be repeated here. Useful features of this methodology are that (1) the detailed spatial variations of the geologic site conditions can be included directly in the calculation of spectral amplitudes, and as will be shown, are one of the significant factors influencing the final result; and (2) the consequences of contributions from large distant earthquakes (in this case from Vrancea in Romania) can also be included. The difference in the results of such an analysis from the old approach based on probabilistic mapping of only peak ground acceleration will become apparent in what follows.

Fig. 1 shows peak design accelerations for Skopje at 0.24 g (for the probability of $p=0.1$ exceedance during an exposure period of $Y=50$ years). Fig. 2 shows the corresponding seismic hazard map, also for the probability of $p=0.1$ exceedance during an exposure period of $Y=50$ years, but calculated by Lee and Trifunac [41]. It gives a 42% larger peak acceleration in Skopje, equal to 0.34 g.

The procedures, which were used for development of the maps in Figs. 1 and 2, are very different and thus the results cannot be compared directly. Nevertheless, the end results suggest differences in peak accelerations in northern Macedonia approaching 50% [40,41]. The amplitudes in Fig. 1 are based on scaling strong motion amplitudes by a weighted combination of five representative empirical-scaling equations for peak ground acceleration and for soil site condition A [59], while the amplitudes in Fig. 2 are for sites on geological basement rock ($s=2$), and on “rock” soil sites ($S_L=0$), and use attenuation equations for scaling Pseudo Relative Velocity Spectra (PSV) based on strong motion recordings in the former Yugoslavia [37,38].

The approach implied for using the peak accelerations in Fig. 1 is that the acceleration at “rock” sites (represented in Fig. 1 by site class A) can be modified to other soil site classes by an analysis of a model that is capable of describing amplification of ground motion. This approach does not consider the geological characteristics of the site. In contrast, we include the effects of site geology via simplified site indicator variables in this paper. In several of our previous studies, we have argued that describing the site conditions in terms of only surface soil properties in the top 30 m (V_{30} or A, B, C, and D, for example) does

Seismic Hazard Map
Macedonia: $A_{max} \sim PSA$ for $T = 0.04$ s - g
Probability of Exceedance = 10.0% in 50 Years

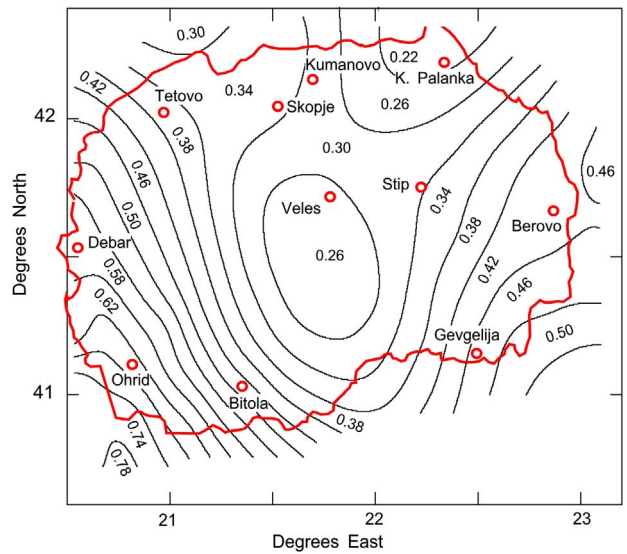


Fig. 2. The upper bound on peak accelerations in terms of $PSA(T) = (2\pi/T)PSV$ (g) computed from UHS at $T=0.04$ s only for local seismicity within 175 km of the site [44]. The contributions from the Vrancea earthquake source in Romania are not included. Contours are shown for a probability of 10% exceedance and an exposure period of $Y=50$ years (redrawn from [41]).

not lead to reliable results and should not be used [39]. In the following, we will work with the description of site parameters that describe site geology ($s=0, 1$, or 2) and site soil properties beyond the depth of 30 m ($S_L=0$ and 1).

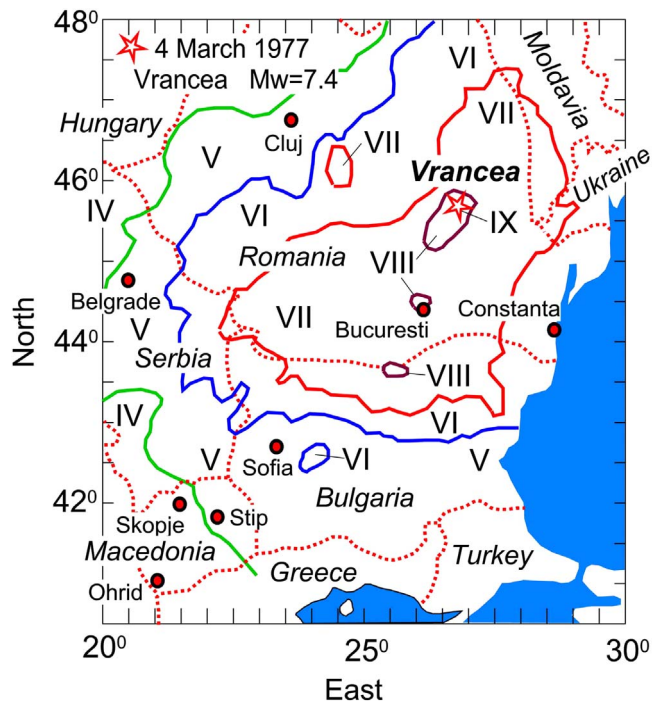


Fig. 3. A map of MKS intensity contours for the March 4, 1977 Vrancea earthquake (extracted from [28]) with $M_w=7.4$, $H=98$ km, and $I_0=IX$. The Vrancea source zone is centered near 45.88° N and 26.98° E in Romania, where intermediate and large earthquakes occur at depths between 60 and 200 km. During this earthquake, the MKS intensity was IV at an epicentral distance of 620 km in Skopje.

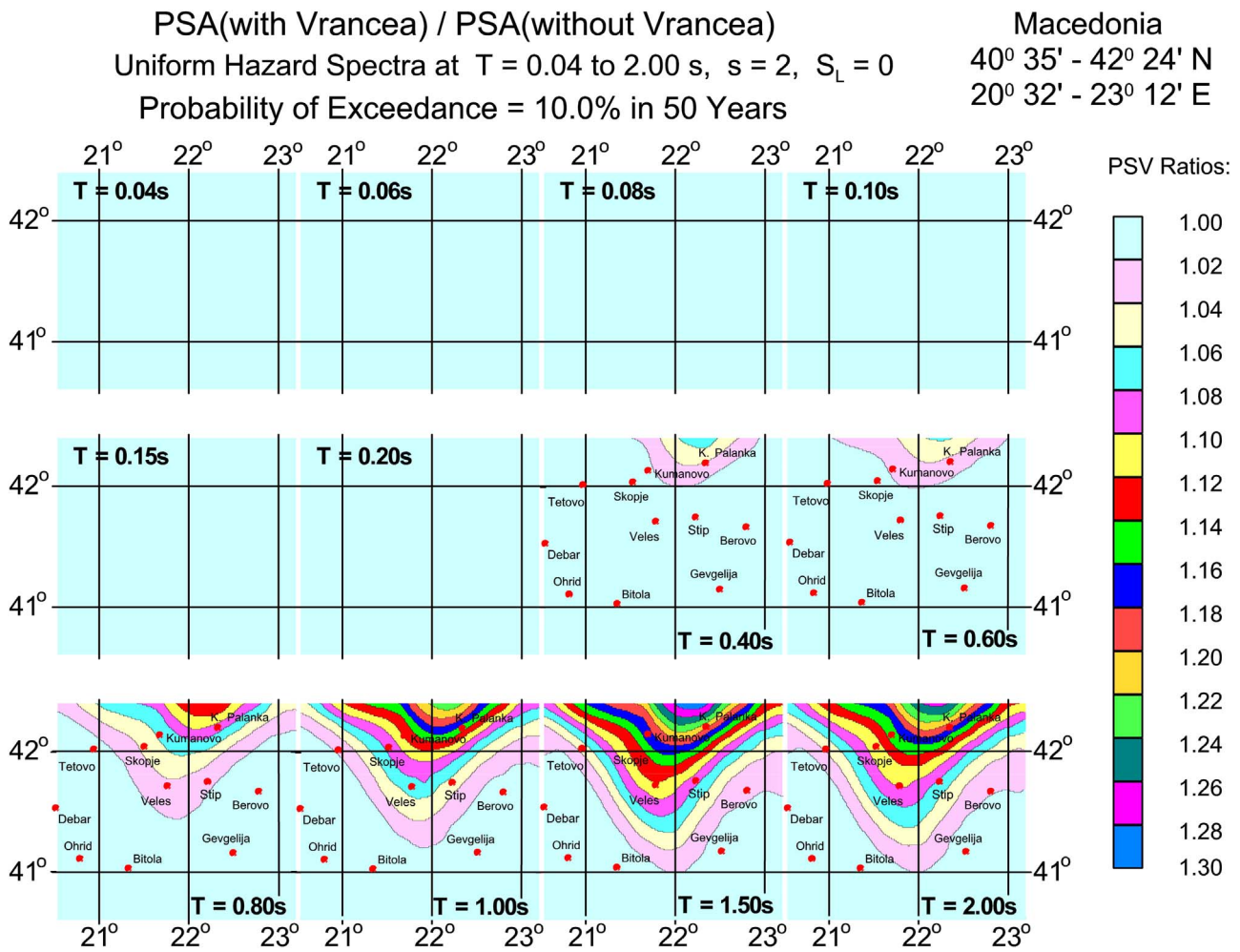


Fig. 4. An example of a seismic zoning map for Macedonia for the Pseudo-Spectral Velocity (PSV) ratio, relative to seismicity without contributions from the Vrancea earthquakes, for a 5% fraction of critical damping. This map is computed with the UHS method for the basement rock sites ($s = 2$) and rock soil sites ($S_L = 0$) at 12 periods ranging from 0.04 to 2.00 s, for a probability of exceedance of $p = 0.10$, and for an exposure time of $Y = 50$ years (redrawn from [41]).

The computation of hazard maps begins with a definition of spatial distribution of seismic sources and their activity in time. The description of this activity, as used in this paper, is briefly outlined in Appendix A. The next step involves the selection of attenuation equations that describe how the desired quantity representing strong ground motion attenuates with distance. The last step combines the contributions of all earthquake sources surrounding the site to compute the distribution functions of the quantity being evaluated at the site and plotting maps to show its spatial distribution.

The performance-based design guidelines in Eurocode 8 [10] (EC8) are expressed by *no-collapse* and *damage limitation* requirements. The no-collapse requirement is associated with a large and hence nonlinear response and the soil-structure system period $T_{nonlinear}$, while the damage limitation requirement is associated with an essentially linear response and hence a shorter equivalent system period, T_{linear} . Because $T_{nonlinear}$ and T_{linear} are associated with different probabilities of exceedance, their respective spectral amplitudes cannot be specified with the same fixed shape spectrum and two corresponding peak accelerations. This is because the spectral shape is different for the different exceedance probabilities [75].

Beyond this, in our previous work we have described other concerns

that point against using the maps like the one shown in Fig. 1. One such concern is that the hazard contributions from large Vrancea earthquakes (Fig. 3) cannot be included when the scaling of design motions is performed via peak accelerations. The reader will find further discussion regarding these concerns in [40,41]. We have shown, for example, that the Vrancea earthquakes dominate seismic hazard in eastern Serbia for intermediate and long periods of strong ground motion. This domination diminishes with increasing epicentral distance, and in northern Macedonia it is down to a range of several to several tens of percents (Fig. 4). In eastern and southwestern Macedonia, this contribution becomes even smaller due to active seismicity in nearby Bulgaria and Albania [41].

Fig. 4 shows the ratio of spectral velocities computed with contributions relative to contributions without the Vrancea earthquakes. It is seen that in the high-frequency range (higher than about 2 Hz), the contribution to spectral velocities from the Vrancea earthquakes can be neglected for all sites in Macedonia for $p=0.1$ and $Y = 50$ years. However, as oscillator periods become longer, these contributions increase. At a frequency of 0.7 Hz, the Vrancea earthquakes increase spectral velocities by about 13% in Skopje. Fig. 4 also shows that south and southwest of the line connecting Tetovo with

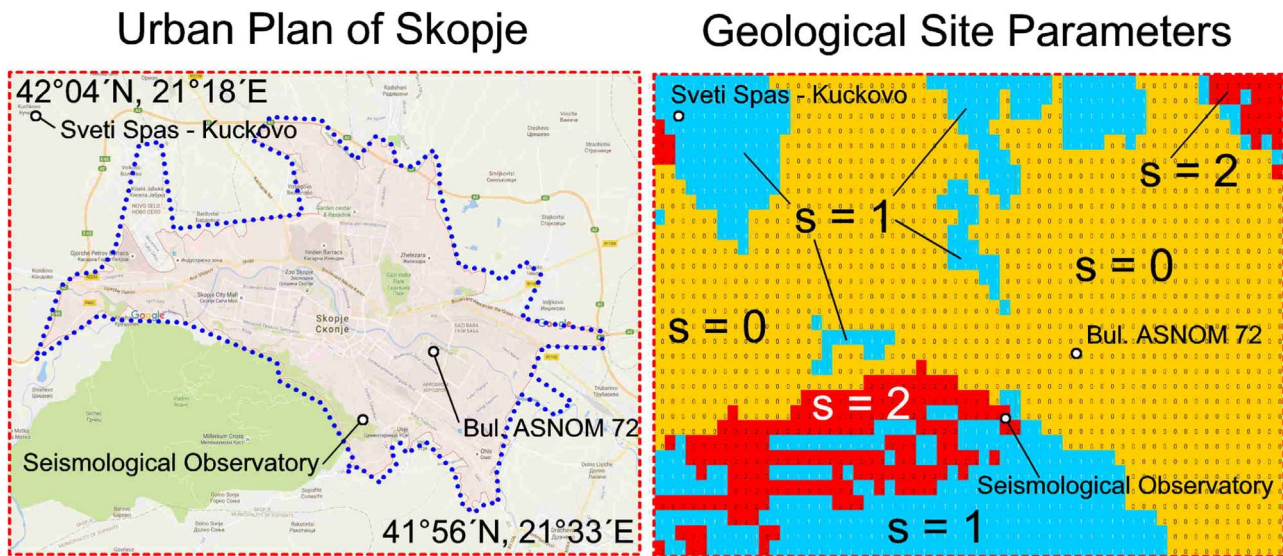


Fig. 5. The General Urban Plan of the City of Skopje (left) and the Geological Site Parameters ($s=0, 1$ or 2) in the same area (right).

Bitola, Gevgelija, and Berovo, at epicentral distances greater than about 600 km, the contribution to seismic hazard from the Vrancea earthquakes can be neglected in Macedonia (Tetovo – 645 km; Bitola – 706 km; Gevgelija – 638 km; Berovo – 578 km). In the areas where local seismic activity is high, the distance of this line to Vrancea decreases, and where the local seismicity is low, this distance increases.

2. Skopje and its surroundings: geological site classification

Geological site classification for computations of Uniform Hazard Spectra for the area surrounding the city of Skopje is based on our interpretation of the geological maps of the Skopje region. There are four basic geological maps that cover the area defined by the “General Urban Plan of the City of Skopje 2012–2022” [9] that include: “Basic geological map – Kačanik” [4]; “Basic geological map – Kumanovo” [5]; “Basic geological map – Skopje” [6]; and “Basic geological map – Titov Veles” [7].

The area that covers the “General Urban Plan of the City of Skopje 2012–2022” (Fig. 5) was divided into cells of 15×15 s in geographic coordinates between $41^\circ 56' N$ and $42^\circ 04' N$; that is, between $21^\circ 18' E$ and $21^\circ 33' E$. For each of the 1920 analyzed cells, the site geology was described by the predominant lithostratigraphic formations and depths.

To determine the geological site condition s for the city of Skopje, we interpreted the description of the site geology that was compiled for each cell. We used the classification methodology proposed by Trifunac and Brady [77] and then classified the site geology for each cell as either “basement rock” ($s=2$), “alluvial and sedimentary deposits” ($s=0$), or “intermediate sites” ($s=1$). The result is shown in the right half of Fig. 5.

3. Mapping seismic hazard via uniform hazard spectra

Mapping the amplitudes of $PSA(T) = 2\pi PSV(T)/T$ where $PSA(T)$ is the Pseudo Absolute Acceleration Spectrum, $PSV(T)$ is the Pseudo Relative Velocity Spectrum, and T is the oscillator period, for $T=0.04$ s gives an upper bound for peak ground acceleration, because in the limit as T tends to zero, $PSA(T)$ tends to peak ground acceleration.

Fig. 6a through d show this upper bound for peak ground acceleration for a 10% exceedance probability, exposure periods of $Y=10$ and 50 years, and for “rock” ($S_L=0$) and stiff ($S_L=1$) soil site conditions. The geological site parameters s are included in the hazard calculations. The spatial variations seen in these figures are dominated by the distance to seismic activity northwest and southeast of Skopje and by the geological site-condition parameters $s=0, 1$ and 2 (see Fig. 5, right). For a 10% exceedance probability and an exposure time of $Y=50$ years, the average peak acceleration in Fig. 6b is consistent with our overall regional estimate [41] of 0.34 g (shown in Fig. 2). For sites on $S_L=1$ (stiff soil), (Fig. 6d) peak accelerations are about 1.7 times larger.

We note that the amplitudes of peak accelerations shown in Fig. 6a through d are not sensitive to the occurrence of distant earthquakes in the Vrancea source zone in Romania. These figures further show that peak accelerations are larger on basement rock ($s=2$) than on intermediate rock sites ($s=1$) by about 30–35% for both “rock” soil sites ($S_L=0$), and for stiff soil sites ($S_L=1$).

In Fig. 6a through d, we emphasize the degree to which peak accelerations (here $PSA(T)$ at $T=0.04$ s approximately represents peak ground acceleration) are dependent on the geological site conditions, and provide a rough basis for comparison with previously published results (e.g., as in Fig. 1). Our results in Fig. 6a through d are not intended for and should not be used in scaling the design spectral amplitudes. Correct scaling of design spectra can be accomplished via microzonation maps based on UHS amplitudes. This will be described in the following section.

4. Contribution of the Vrancea earthquakes to seismic hazard

The Vrancea earthquakes occur at a large epicentral distance from Skopje (620 km; Fig. 3). This results in attenuation of high-frequency spectral amplitudes [46,47], thus the UHS for peak accelerations in Skopje (Fig. 4), for a typical range of exceedance probabilities, are dominated only by local seismicity. Consequently, for typical hazard mapping of peak accelerations in Skopje, the Vrancea earthquakes can be ignored. However, this is different in cases of intermediate and long-period spectral amplitudes, in which the Vrancea earthquakes (with

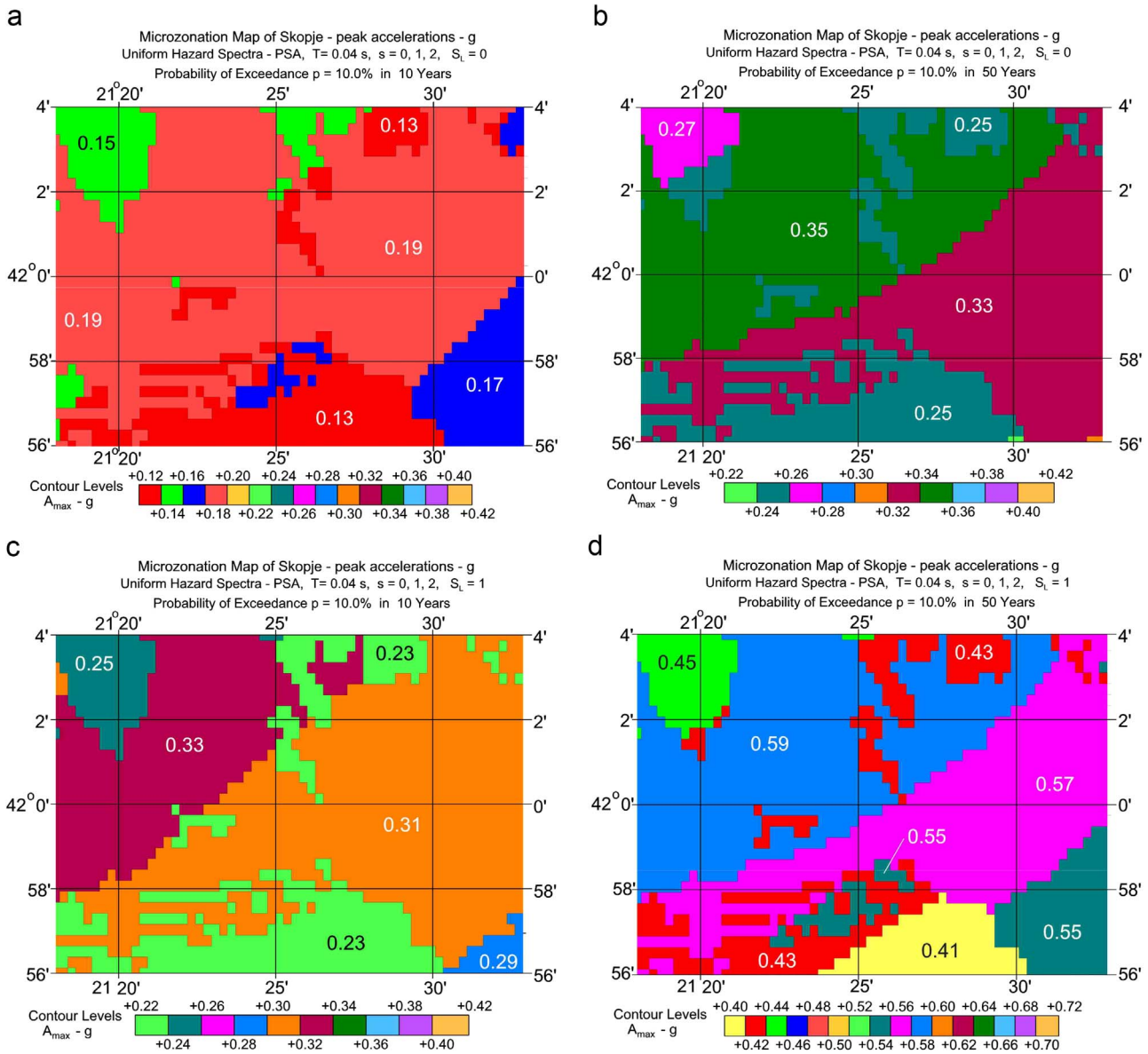


Fig. 6. a. The upper bound for peak ground acceleration in Skopje in terms of PSA(0.04) (with units of g), at “rock soil” sites, $S_L = 0$, for a 10% exceedance probability and an exposure period of $Y = 10$ years. 6b. The upper bound for peak ground acceleration in Skopje in terms of PSA(0.04) (with units of g), at “rock soil” sites, $S_L = 0$, for a 10% exceedance probability and an exposure period of $Y = 50$ years. 6c. The upper bound for peak ground acceleration in Skopje in terms of PSA(0.04) (with units of g), at stiff soil sites, $S_L = 1$, for a 10% exceedance probability and an exposure period of $Y = 10$ years. 6d. The upper bound for peak ground acceleration in Skopje in terms of PSA(0.04) (with units of g), at stiff soil sites, $S_L = 1$, for a 10% exceedance probability and an exposure period of $Y = 50$ years.

$M > 6.5$) contribute progressively more as periods of ground motion become longer.

Fig. 7a through d show contours of PSA($T = 1.0$ s) with contributions from the Vrancea earthquakes. The Vrancea earthquakes contribute appreciably at $T = 1.0$ s and longer periods, and this contribution progressively increases in Macedonia as one moves northeast [41]. The contribution of the Vrancea sources is also larger where the local seismicity is relatively low (see Appendix A and Fig. 4).

A detailed comparison of Fig. 7a through d, with Fig. 6a through d, shows that the spatial distribution of short- and long-period amplitudes is different. This is caused by a variable shape of UHS and by the contribution to long-period spectral amplitudes from the Vrancea sources with strong motion waves arriving from the northeast. Fig. 8a

and b also show this in terms of the ratios of spectral amplitudes computed relative to the spectral amplitudes without the contribution from the Vrancea sources.

The relative increase of spectral amplitudes due to the contribution from the Vrancea earthquakes, shown in Fig. 8a and b, is the same for all comparisons among the same geological site conditions ($s = 0, 1$ and 2) and for the oscillator period shown ($T = 1.00$ s). However, the amplitudes of PSA($T = (2\pi/T)PSV$ for UHS at $T = 1.00$ s at sites on intermediate rocks ($s = 1$) will be larger than at the basement rock sites ($s = 2$). In this paper, we show most results for the sites on geological basement rock ($s = 2$) to facilitate qualitative comparisons with previously published results, which are typically shown only for the type A sites.

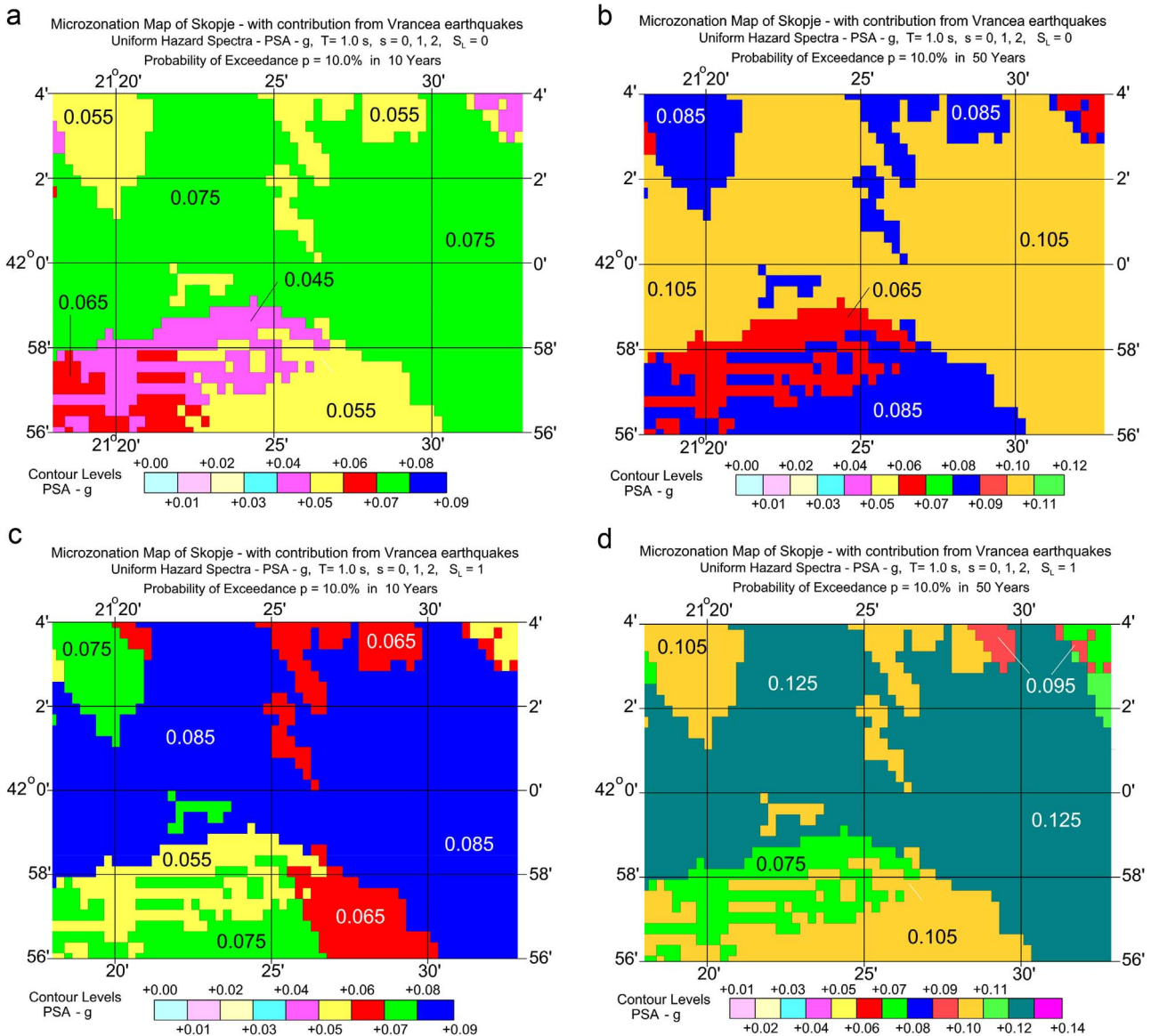


Fig. 7. a. Uniform Hazard Acceleration Spectra, $PSA(T)$, with units in g, at $T = 1$ s, at “rock” soil sites, $S_L = 0$, for a 10% exceedance probability and an exposure interval of $Y = 10$ years. 7b. Uniform Hazard Acceleration Spectra, $PSA(T)$, with units in g, at $T = 1$ s, at “rock” soil sites, $S_L = 0$, for a 10% exceedance probability and an exposure interval $Y = 50$ years. 7c. Uniform Hazard Acceleration Spectra, $PSA(T)$, with units in g, at $T = 1$ s, at stiff soil sites, $S_L = 1$, for a 10% exceedance probability and an exposure interval of $Y = 10$ years. 7d. Uniform Hazard Acceleration Spectra, $PSA(T)$, with units in g, at $T = 1$ s, at stiff soil sites, $S_L = 1$, for a 10% exceedance probability and an exposure interval of $Y = 50$ years.

5. Practical determination of UHS

Design spectra based on UHS at a building site can be calculated for each site-specific condition, but these calculations are time-consuming and require detailed knowledge to select the required scaling parameters. A simple, convenient alternative for engineering applications, which Lee and Trifunac introduced in the mid-1980s, is to prepare maps of UHS for given response periods, site conditions, probabilities of exceedance and exposure time, and then to read the spectral amplitudes from the map contours. The report, “Microzonation of a Metropolitan Area,” describes this procedure and presents examples of how it can be carried out [36]. Fig. 9a through d show examples of such maps for $\log_{10}PSV(T)$ amplitudes at 12 periods ranging from 0.04 s to 2.00 s, for 5% damping, horizontal motions, at “rock” and stiff soil sites ($S_L = 0$ and 1), for 10% exceedance probability and exposure periods of $Y = 10$ and

50 years. By reading the spectral amplitudes at the given location, the UHS of PSV can be constructed by interpolating the values read from the 12 periods. Examples of such interpolations are illustrated in Figs. 10a through 12b, at three sites with different geological and soil site parameters (Skopje -Seismological Observatory, with $s = 2$ and $S_L = 0$; Kuckovo, with $s = 1$ and $S_L = 1$; and Bul. ASNOM 72 with $s = 0$ and $S_L = 1$).

Figs. 10a through 12b all show a monotonically increasing contribution from the Vrancea sources as the oscillator periods become longer. Overall, long-period spectral amplitudes tend to be larger on $s = 0$ and $S_L = 1$ (Bul. ASNOM 72, Fig. 12a,b) than on $s = 2$ and $S_L = 0$ (Skopje-Seismological Observatory, Fig. 10a,b), provided the sites are at comparable distance from large Vrancea sources, as those are in these examples.

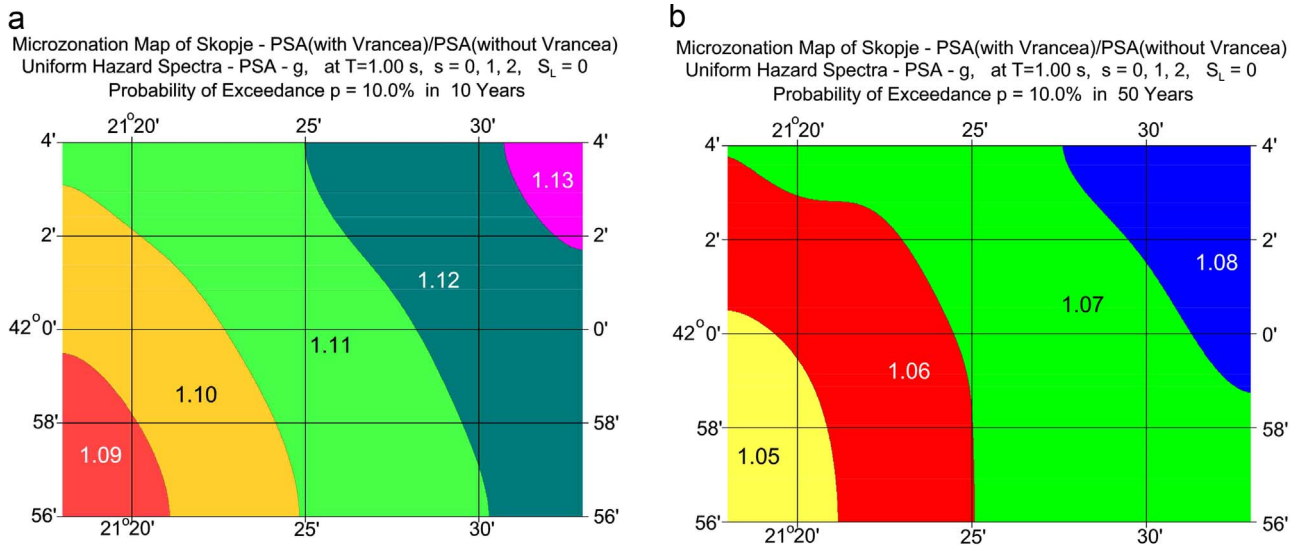


Fig. 8. a. Ratio of Uniform Hazard Acceleration Spectra, $PSA(T)$, with and $PSA(T)$ computed without contributions from the Vrancea earthquakes at $T = 1$ s, at “rock” soil sites, $S_L = 0$, for a 10% exceedance probability and an exposure interval of $Y = 10$ years. 8b. Ratio of Uniform Hazard Acceleration Spectra, $PSA(T)$, with and $PSA(T)$ computed without the contribution from the Vrancea earthquakes at $T = 1$ s, at “rock” soil sites, $S_L = 0$, for a 10% exceedance probability and an exposure interval of $Y = 50$ years.

6. Discussion and conclusions

We have shown that the regional variations of UHS amplitudes over an area of a large city are considerable. In the examples shown for Skopje, three main sources of these variations are (1) the geological and soil site conditions; (2) the local seismicity mainly northwest and southeast of Skopje; and (3) the contributions from large Vrancea earthquakes at a distance of about 620 km northeast in Romania. The different effects of distance from these sources can clearly be seen in Fig. 6a through d (along the northwest to southeast direction), 7a through 7d, and 8a and 8b (along the southwest to northeast direction), and 9a through 9d (at short and long periods, respectively). Relative contributions from these sources will differ for different cities depending on the degree to which geologic and soil site conditions vary, as well as on the relative strength and direction towards local versus distant earthquakes.

In Macedonia, peak accelerations are dominated by the local seismic activity almost everywhere, and the contribution from the Vrancea earthquakes can be neglected. However, in central and northeastern parts of the country, the intermediate- and long- period spectral amplitudes are affected by contributions from large Vrancea earthquakes. These contributions are small in the southwestern, southern and southeastern areas of Macedonia, and strongest in the northcentral and northeastern areas (Fig. 4; [41]).

In all calculations throughout this paper we have used empirical scaling equations that were developed from the strong motion data recorded in the former Yugoslavia [37,38,47]. This data had only a few recordings at deep soil sites ($S_L = 2$, [71,73]) and hence our empirical scaling equations for PSV amplitudes do not include the scaling in terms of $S_L = 2$. Therefore, examples including $S_L = 2$ are not included in this paper. For future engineering design at $S_L = 2$ sites, additional analyses will be required to modify UHS presented in this paper to approximate the amplitudes expected at $S_L = 2$ sites. This can be based on several other scaling equations that do include $S_L = 2$ scaling factors in California and elsewhere [33].

It is noted that all results presented in this paper are only of

preliminary nature. Our scaling models for strong ground motion in former Yugoslavia are more than 20 years old and based on strong motion data, which was recorded up to the early 1980s [23,78,79]. Our model for scaling of PSV spectra in Serbia [47]—that we are also using in the case of Macedonia in this paper for earthquakes in the Vrancea source zone—was published recently. However, that paper only includes large Vrancea earthquake data and has not been tested by comparison with recorded data in Macedonia, because strong motion data is not available at epicentral distances of about 600 km.

More detailed and comprehensive analyses of this kind will be possible only when new and abundant recordings of local and Vrancea earthquakes become available. In the meantime, it is hoped that this analysis will help motivate and guide observational programs that will contribute such needed data. Furthermore, it is hoped that our profession will abandon outdated and ill-founded methods of scaling strong motion amplitudes, as in, for example, using site classifications in terms of V_{50} or A, B, C, and D, [39], and finally realize that scaling the design spectra by peak acceleration and fixed shape spectra is not conservative.

In this paper we have used the seismicity model in which earthquakes are represented by points. Such representation works well and is sufficiently accurate for general purpose mapping of seismic hazard, but it is not suitable for modeling seismic hazard for important structures (nuclear power plants, dams, major bridges, major pipe lines, and alike) when active faults are within several source dimensions from the site. When this occurs, detailed three-dimensional representation of earthquake sources must be performed so that the specific geometric relationships can be included in the attenuation models and UHS calculations. The point source representation via UHS method is also not meant for use when predicting spatial site amplification for a given nearby earthquake. For example, the 1963 Skopje earthquake produced specific distribution of strong motion amplitudes that resulted in the specific distribution of damaged buildings. This distribution was in turn influenced by the specific direction of wave arrival and subsequent interference and amplification through the three-dimensional geological structure (Appendix C). An earthquake with a different azimuth of

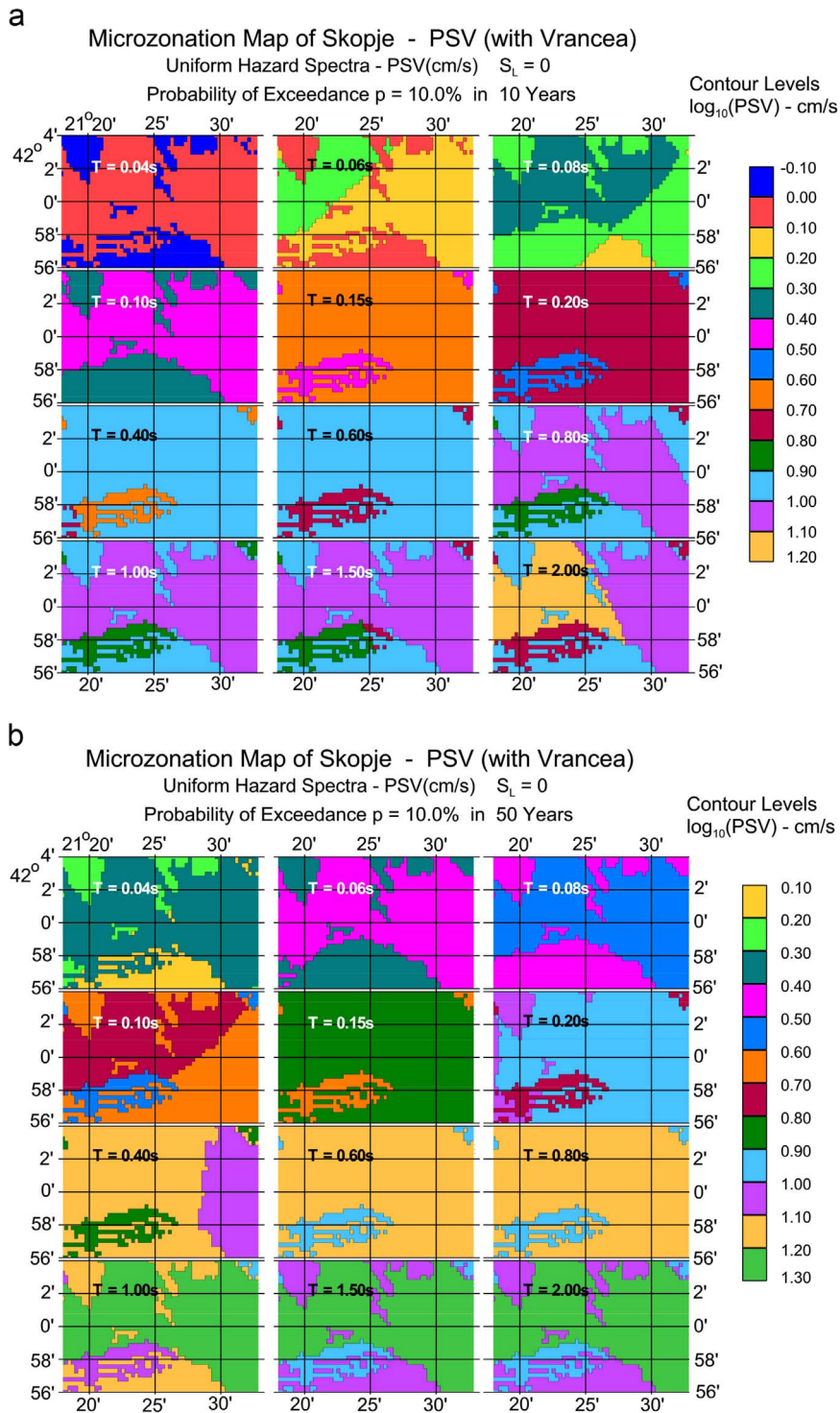


Fig. 9. a. An example of a Skopje seismic microzonation map for Pseudo Spectral Velocity (PSV) with a 5% fraction of critical damping. The map is computed with the UHS method for local seismicity and the contribution from the Vrancea earthquakes combined at the “rock” soil sites ($S_L = 0$), at 12 periods ranging from 0.04 to 2.00 s, for exceedance probability of $p = 0.10$ and an exposure time of $Y = 10$ years. 9b. An example of a Skopje seismic microzonation map, for Pseudo Spectral Velocity (PSV), with a 5% fraction of critical damping. The map is computed with the UHS method for local seismicity and the contribution from the Vrancea earthquakes combined at the “rock” soil sites ($S_L = 0$) at 12 periods ranging from 0.04 to 2.00 s, for an exceedance probability of $p = 0.10$ and an exposure time of $Y = 50$ years. 9c. An example of a seismic microzonation map for Skopje, for Pseudo Spectral Velocity (PSV), for a 5% fraction of critical damping. The map is computed using the UHS method for local seismicity and the contribution from the Vrancea earthquakes combined, at stiff soil sites ($S_L = 1$), at 12 periods ranging from 0.04 to 2.00 s, for an exceedance probability of $p = 0.10$ and an exposure time of $Y = 10$ years. 9d. An example of a seismic microzonation map for Skopje, for Pseudo Spectral Velocity (PSV), for a 5% fraction of critical damping. This map is computed with the UHS method for local seismicity and contribution from the Vrancea earthquakes combined, at the stiff soil sites ($S_L = 1$), at 12 periods ranging from 0.04 to 2.00 s, for an exceedance probability of $p = 0.10$ and an exposure time of $Y = 50$ years.

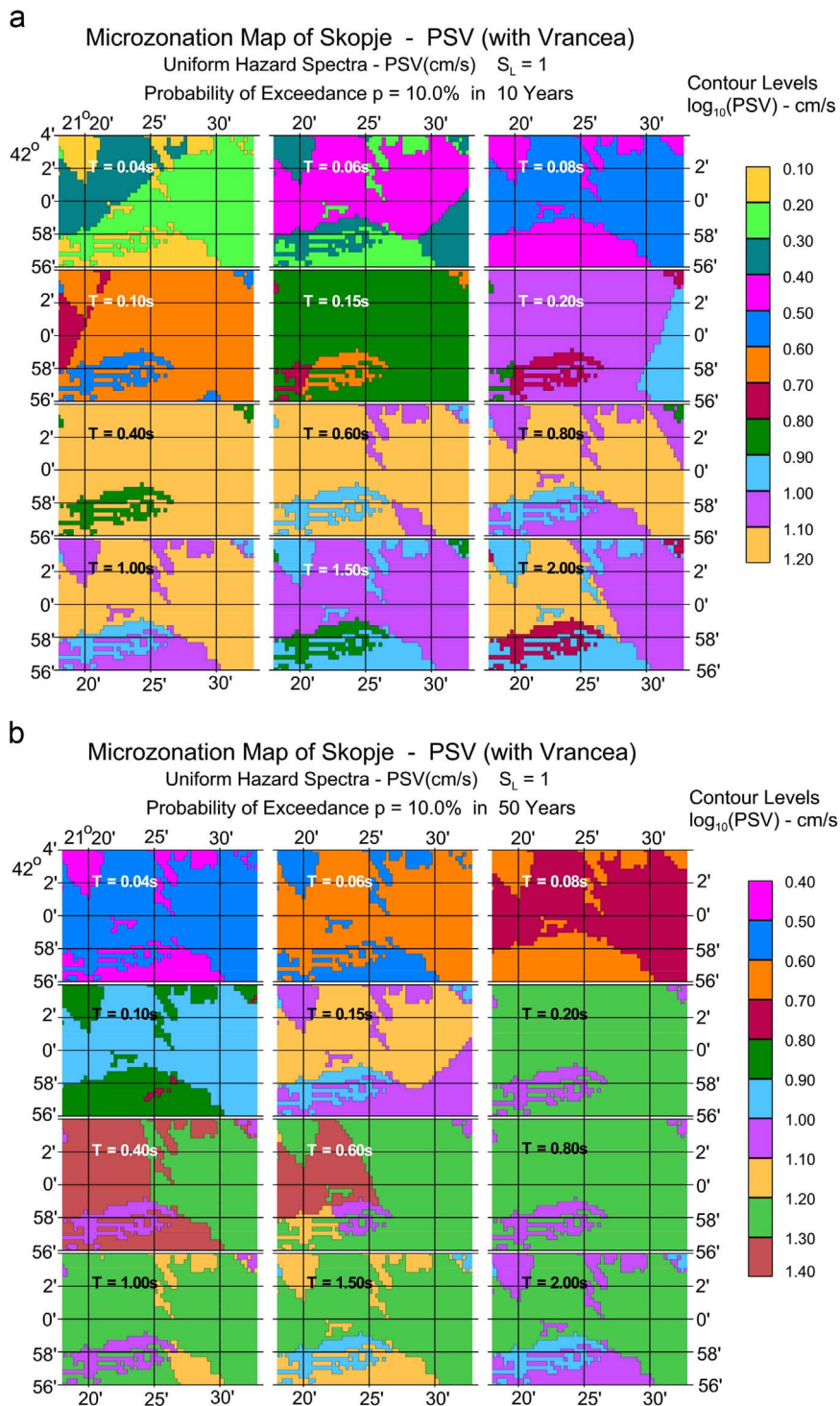


Fig. 9. (continued)

arriving waves will produce a very different pattern of strong motion amplitudes and the resulting damage to structures. The spatial variations of strong motion amplitudes that we illustrate in this paper represent a distribution based on all possible outcomes from a very large number of such individual events, which are then described by the UHS amplitudes. A scenario for which a deterministic description of

design spectra will offer a more robust representation of what can be expected from future earthquakes would be the case in which the hazard is dominated by a single well-known fault system, of which enough is known to be able to predict the largest possible events and its all other governing variables.

A refinement of the point source representation is possible in terms

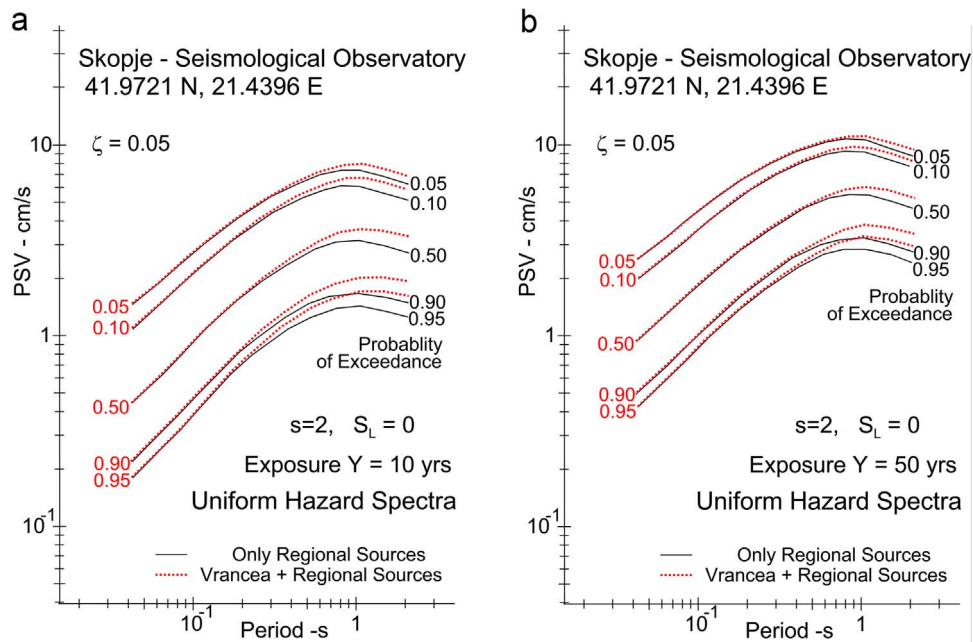


Fig. 10. a. A comparison of Uniform Hazard Spectra of PSV at the Skopje-Seismological Observatory for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania, for an exposure time of $Y = 10$ years. 10b. A comparison of Uniform Hazard Spectra of PSV at the Skopje-Seismological Observatory for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania, for an exposure time of $Y = 50$ years.

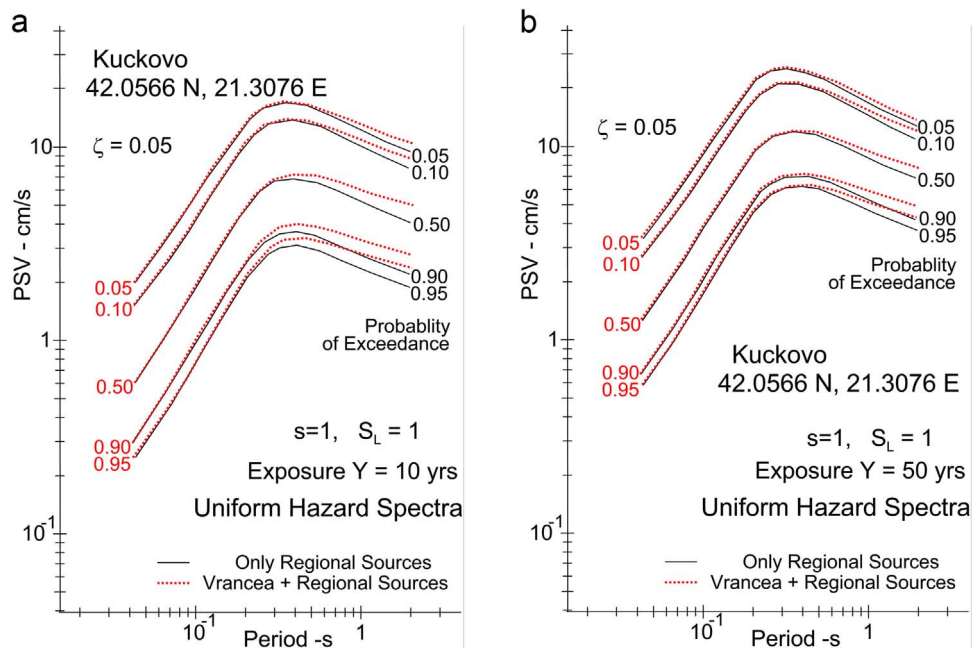


Fig. 11. a. A comparison of Uniform Hazard Spectra of PSV at Kuckovo for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania for an exposure time of $Y = 10$ years. 11b. A comparison of Uniform Hazard Spectra of PSV at Kuckovo for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania for an exposure time of $Y = 50$ years.

of line sources [43] when such information is available to accompany seismic activity rates and data on maximum magnitudes. This information was not available in sufficiently detailed and complete form for the seismicity surrounding Skopje and was therefore not used in the examples presented in this paper.

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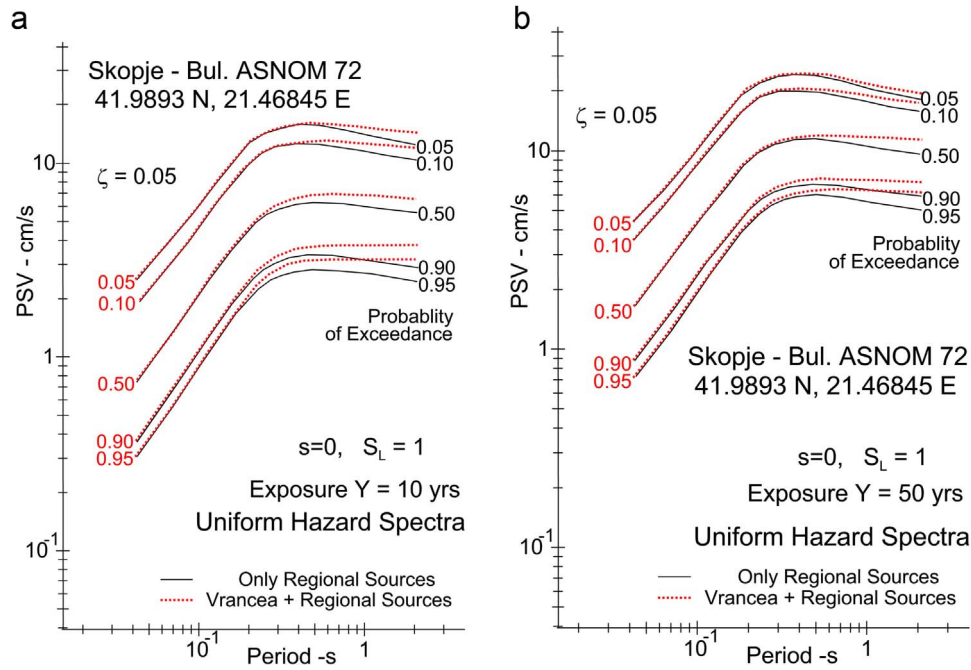


Fig. 12. a. A comparison of Uniform Hazard Spectra of PSV at Skopje – Bul. ASNOM 72, for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania for an exposure time of $Y = 10$ years. 12b. A comparison of Uniform Hazard Spectra of PSV at Skopje – Bul. ASNOM 72, for seismic activity with (dashed lines) and without (solid lines) contributions from the Vrancea earthquakes in Romania for an exposure time of $Y = 50$ years.

Appendix A. – Seismic activity and seismicity model

To illustrate computations for the strong motion earthquake hazard, we briefly summarize the data on seismic activity surrounding investigated locations and procedures for deriving the relevant seismicity model. The earthquake activity in the region is assumed to be well represented by a catalog compiled by merging records listed in the following catalogs:

- the BSHAP2 catalog (510 BCE–2012) that was compiled during the BSHAP2 NATO-funded project by seismologists from Albania, Croatia, Macedonia, Montenegro, Serbia, and Turkey [49];
- the ISC-catalog (2013–2014) (<http://www.isc.ac.uk>).

If not reported as such, all magnitudes were converted to moment magnitudes (M_w) using regionally adjusted regressions between M_w on one side and M_s , M_L , or m_b on the other [49]. The catalog was declustered using time-space windows, the size of which depended on the mainshock's magnitude as described, for example, in Herak et al. [17], thus removing dependent events (foreshocks and aftershocks). In estimating recurrence parameters, only mainshocks with magnitude exceeding 3.4 were considered. Fig. A1 shows the geographical distribution of the earthquake epicenters.

Seismic activity is described by the earthquake occurrence rate in terms of moment magnitude ($M = M_w$), assuming the validity of the truncated Gutenberg-Richter recurrence relation:

$$N(M) = \begin{cases} 10^{a-bM} & M_{\min} \leq M \leq M_{\max} \\ 0 & \text{otherwise} \end{cases}, \quad (\text{A.1})$$

where $N(M)$ is the number of events with magnitudes greater than or equal to M , and $M_{\min} \leq M \leq M_{\max}$ is the allowable range of magnitudes. M_{\min} varies in space and time according to the completeness of the contributing catalog(s), and the distribution of M_{\max} (Fig. A2) is assumed by taking into account the magnitudes and intensities of the largest historical earthquakes and the lengths of the known major fault segments. The spatial and temporal completeness of the catalog (see Fig. A3 for examples) was estimated as proposed by Herak et al. [17] (for details, see also [18,19]).

The seismicity of the region was modeled using a variant of the distributed smoothed seismicity approach [e.g., Frankel [12] and Frankel et al. [13]; see also Lapajne et al. [29] who applied it to model the seismicity of Slovenia]. This method was also used to compile the earthquake hazard maps for Croatia [16], which are adopted as base maps in the National Annex to EC8 [20]. For computations, the region is divided into a mosaic of rectangular cells ($0.1^\circ \times 0.1^\circ$, or approximately $11.1 \times 11.1 = 123 \text{ km}^2$). For each cell, parameters a and b in Eq. (A.1), along with their uncertainties, are calculated taking the magnitude completeness thresholds into account. Parameter b is estimated using the maximum-likelihood algorithm of Weichert [84], which considers only earthquakes above their respective completeness thresholds within the smallest circle centered in each of the cells that holds at least 40 such events. The resulting spatial distribution of the b -value is shown in Fig. A2. a is assessed by counting the number of events $N_1 = N(M \geq 3.5)$, $N_2 = N(M \geq 3.8)$, $N_3 = N(M \geq 4.2)$ within the circle that occurred after the corresponding onset of complete reporting. For each N_i , $N_{0i}(b) = 10^a$ is estimated using Eq. (A.1), and representative a is obtained by taking the logarithm of the average. The seismicity rates thus obtained are normalized to one year and to an area of $10,000 \text{ km}^2$. After a -values were assigned to each of the grid-cells, the resulting spatial distribution is smoothed using a bivariate, normal-elliptical smoothing kernel (for an example, see e.g., [29]), with the major axis directed along the predominant fault strike within the corresponding source zone whenever it is known. Here we extracted such information from the database of

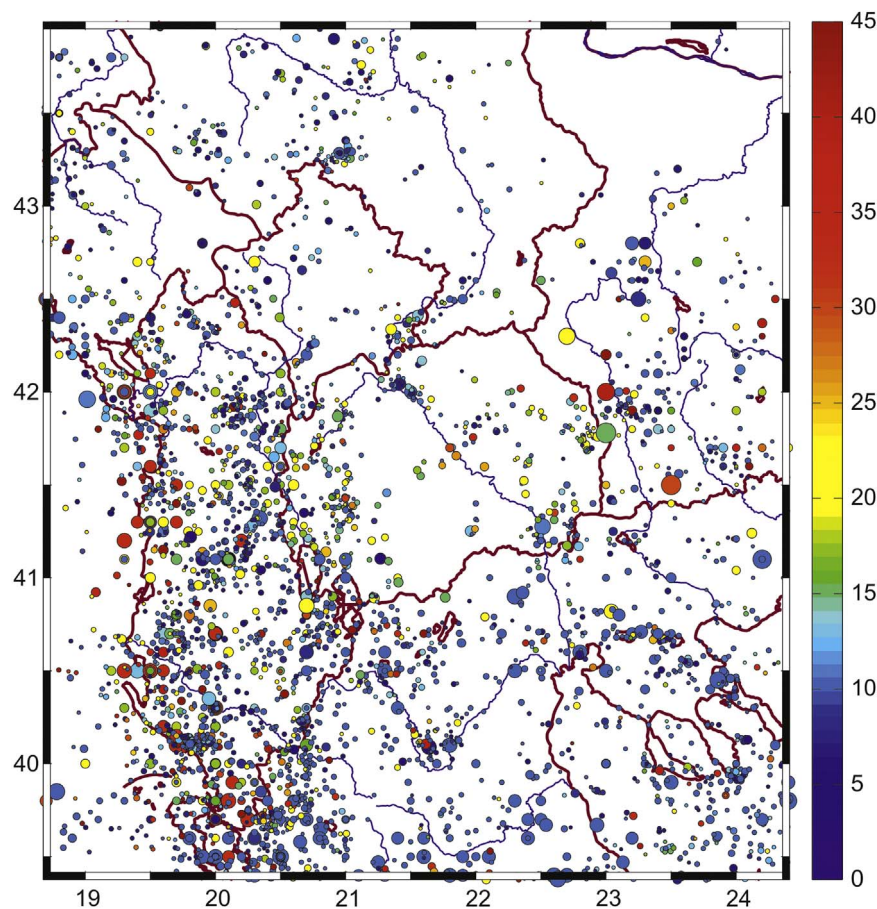


Fig. A1. Earthquake epicenters (main shocks only, $M \geq 3.4$). The colors indicate focal depths according to the color scale on the right. The size of the symbol's scales is according to magnitude.

seismogenic faults, which is one of the products of the SHARE-project ([8], <http://diss.rm.ingv.it/share-edsf/>) that also lists the corresponding predominant style of faulting (normal, reverse, strike-slip). If the predominant strike was unknown, circular Gaussian smoothing was applied. The widths of Gaussian distribution (standard deviations) along the major and minor axes are scaled to the expected maximum fault length and to the width of the surface projection of the fault plane, respectively, estimated for the corresponding M_{max} in the cell by relations of Wells and Coppersmith [85]. In this way, the seismicity potential estimated on the basis of past earthquakes that occurred in the neighborhood of each of the grid-cells, is distributed along the known faults systems. The seismicity model is then defined for each of the cells by the following parameters:

- geographical coordinates of the cell center,
- a -value, and its standard deviation,
- b -value, and its standard deviation,
- maximum moment magnitude, M_{max}
- average focal depth (km), and its standard deviation,
- predominant strike of the seismogenic faults ($^{\circ}$), and its standard deviation,
- predominant style of faulting (unknown, normal, reverse, strike-slip).

Our seismicity model is mostly based on the past seismicity record. It does not explicitly consider fault sources, as we feel that necessary data on positions of seismogenic faults, their segmentation, geometry, Quaternary activity rates, and so forth, are still far from reliably known and complete. Nevertheless, as previously noted, the model includes some of the fault-specific data (predominant strike, the style of faulting, lengths of some known segments) when such data were available.

Appendix B. – Seismicity of the Vrancea source zone

The Vrancea earthquakes in Romania occur near a sharp bend of the southeastern Carpathians [21]. The seismicity is concentrated in a high-velocity focal volume in the depth range from about 60–200 km [53,61]. At shallower depths (0–60 km), earthquakes occur sporadically and are not large, with magnitude typically below 5.5. Source properties of large Vrancea earthquakes and the geology of the surrounding areas generally lead to elongation of the intensity contours toward the southwest and Serbia and Macedonia. This is illustrated in Fig. 3 by the intensity contours of the 1977 earthquake [28]. The cities in Macedonia were at considerable epicentral distances from this Vrancea earthquake, from 550 to 730 km (Stip – 598;

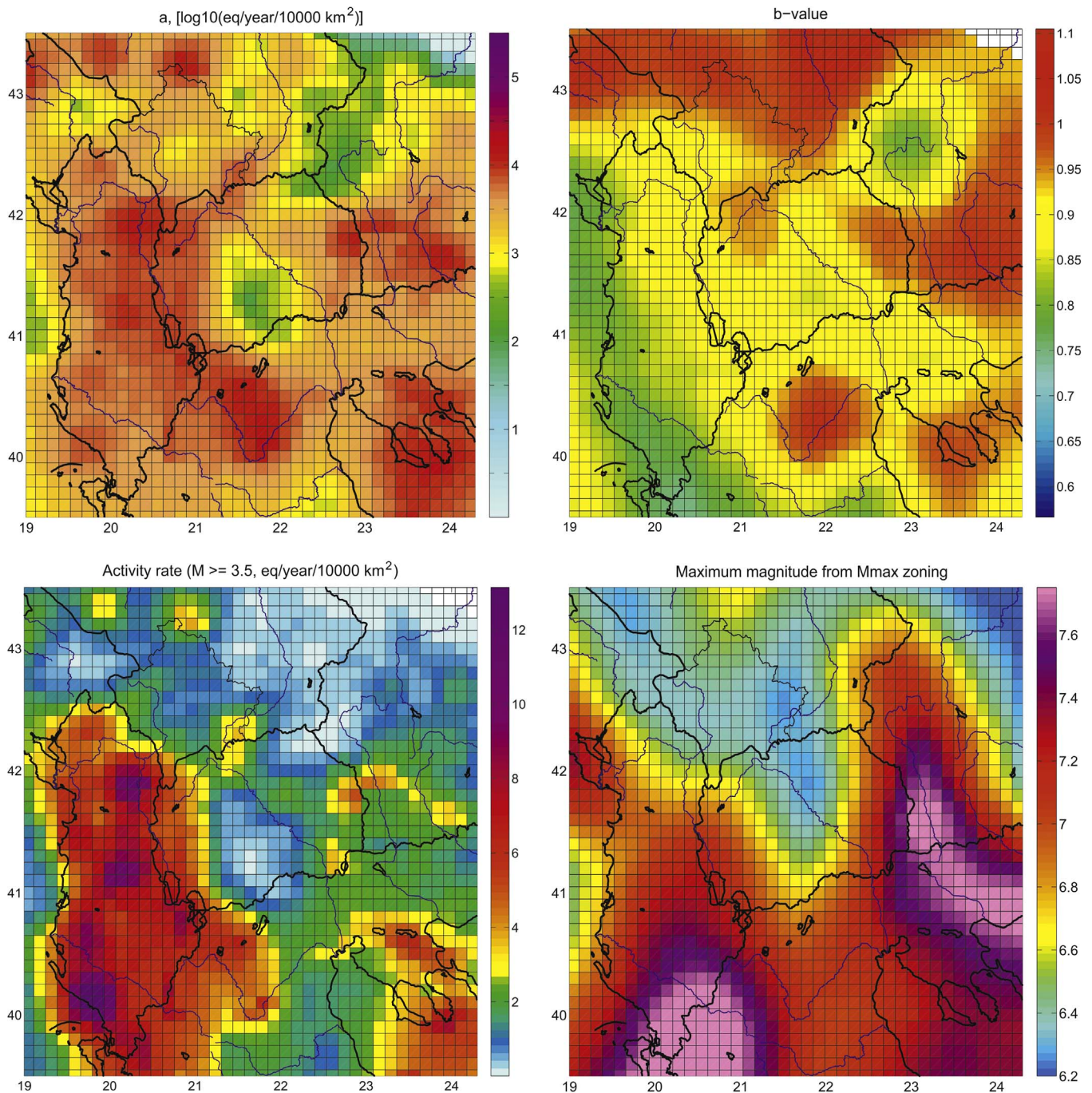


Fig. A2. Parameter spatial distributions of the seismicity model. *Top left:* the a -value in the Gutenberg-Richter relation (normalized to 10,000 km² and one year); *Top right:* the b -value in the Gutenberg-Richter relation; *Bottom left:* the activity rate for $M \geq 3.5$ (the number of events normalized to one year and 10,000 km²); *Bottom right:* maximum magnitude, M_{max} .

Skopje – 619; Ohrid – 726 km). In the March 4, 1977 earthquake, Stip was in zone V (MSK scale), while Skopje and Ohrid were in zone IV.

Historical data on Vrancea earthquake occurrences have been compiled by Radu [57] and Purcaru [55]. The corresponding Gittenberg-Richter trend $\log_{10}N = a - bM$, which describes the number of earthquakes N per year greater or equal to a magnitude M , has been studied by many authors. Here we mention Radulian et al. [56] who give $a = 4.77 \pm 0.24$ and $b = 0.89 \pm 0.04$; Wenzel et al. [83] and Oncescu et al. [51] who give $a = 4.10$ and $b = 0.78$; and Paskaleva [52] who give us $a = 4.21$ and $b = 0.80$. The associated estimates of the largest possible magnitude for the Vrancea events range from 7.8 to 8.1. In Fig. B1 we also reproduce the observed values of N versus M for the period between 1980 and 1997 (redrawn from [51]). For the calculations in this paper, we assumed $a = 4.10 \pm 0.24$, $b = 0.78 \pm 0.04$, and that M_{max} is uniformly distributed between 7.9 and 8.1.

Appendix C. – 1963 Skopje Earthquake

To provide additional background on seismic activity in Skopje, Macedonia, we include this appendix and selected references that describe the effects of the 1963 earthquake. Of particular interest is the report prepared by the UNESCO team of experts, who visited the area and reported on their findings in “The Skopje Earthquake 1963” [82].

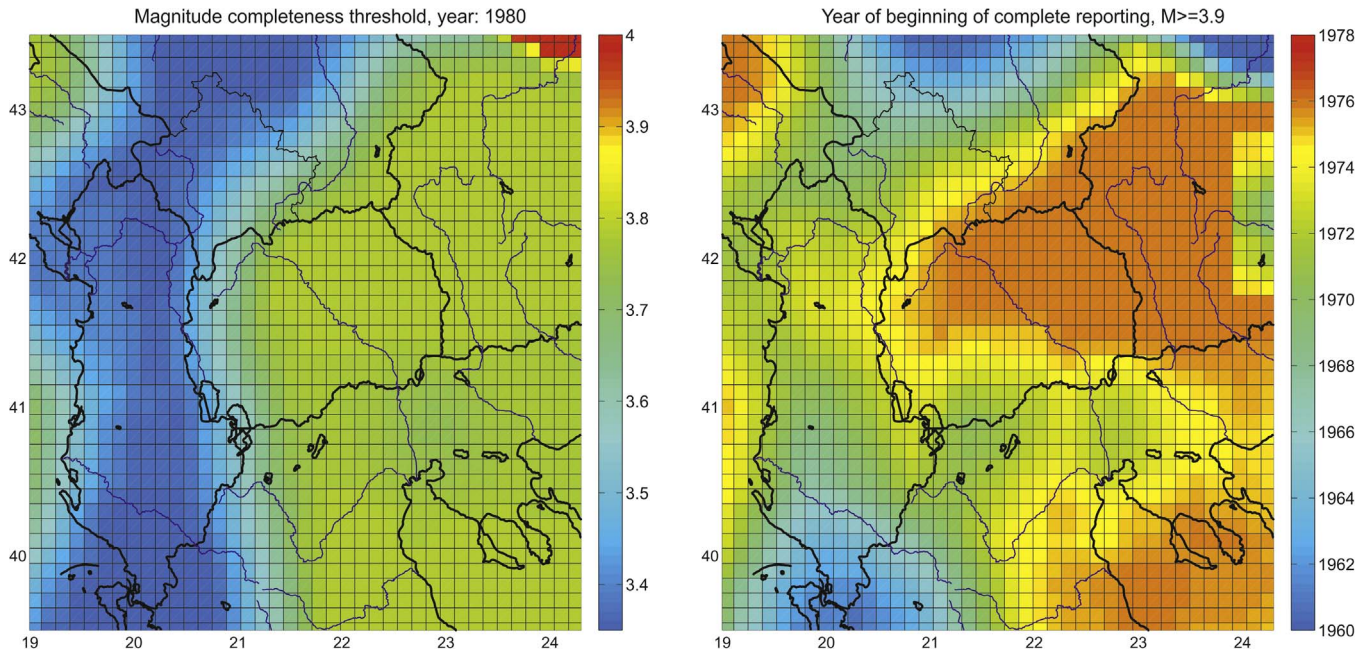


Fig. A3. Examples of the catalog’s completeness analyses. *Left*: the magnitude completeness threshold for the year 1980; *Right*: the beginning year of complete reporting for magnitudes ≥ 3.9 .

The earthquake of July 26, 1963 was felt over an area of 200,000 square kilometers. It killed 1070 people, wounded 3,330, and caused major damage. The epicentral intensity was IX on the MCS scale, it released energy of 10^{21} ergs, and its magnitude was reported as $M = 6.1$. There were no strong motion accelerographs operating at the time so the amplitudes and time characteristics of strong motion were not recorded. The focal mechanism was associated with NNW-SSE dilatation and ENE- WSW compression [87].

By an inverse analysis of ground motions, and using simulations based on simple rectangular fault and converting the computed ground motion velocities into intensities, Suhadolc et al. [60] described the best fit of the reported intensities. These were associated with a rectangular fault 15×8 km, with strike 298° , dip 79° , and rake 22° , with the top of the fault at 2 km below ground surface, a nucleation point at 9-km depth that spread bilaterally from focus, and with $M = 5.9$. The position of the horizontal projection of their fault model is shown in Fig. C1 by dashed lines.

The zones marked I through IV in Fig. C2 outline the areas with different degrees of building damage [54,86]. The heaviest damage occurred in the city center and south of the Skopje Zoo (see the two zones of heavy damage in Fig. C1 marked as I). Most of the buildings in the new city (south of river Vardar) were of brick masonry type, with heights up to 6 stories. Most of the destroyed buildings were of this type.

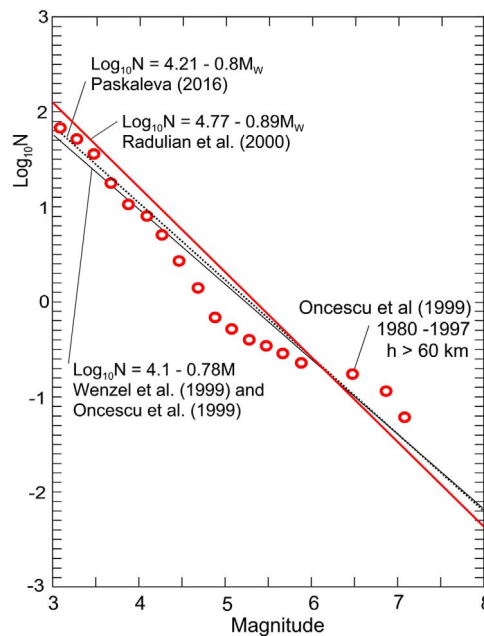


Fig. B1. The number of earthquakes greater or equal to M per year in the Vrancea source zone in Romania.

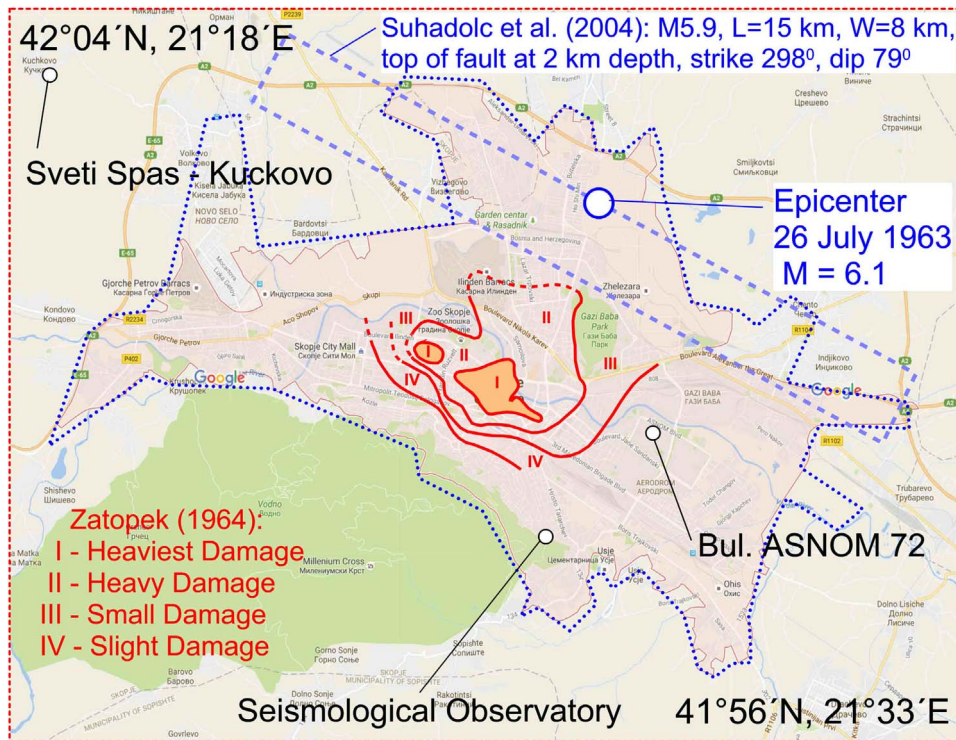


Fig. C1. A horizontal projection of the proposed fault plane for the July 26, 1963 earthquake (dashed rectangle, Suhadolc 2004), earthquake epicenter, and the areas of damaged buildings in Skopje [86].

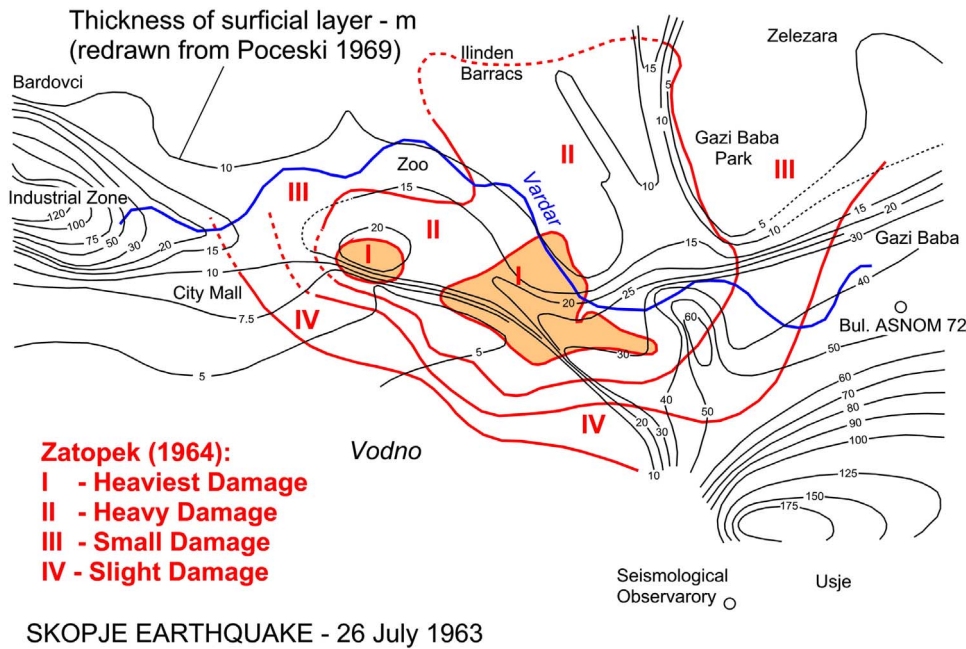


Fig. C2. The distribution of damaged buildings and of the depth (m) of soil layer (redrawn from [54]) shows that the heaviest damage took place where layer thickness drops from 15 to 20 m down to 7.5 and 5 m.

Poceski [54] noted that the zone of heaviest damage coincides with an abrupt change of the soil layer thickness from about 20 m down to 5–7.5 m. It appears that the waves arriving from north-northeast were amplified by a progressive decrease in the layer thickness and by interference with reflected waves from the abrupt end of the layer. This type of amplification of strong ground motion is common and has resulted in increased damage during many other earthquakes (e.g., [80,11,22]).

¹ Can be downloaded from <http://home.iitk.ac.in/~vinaykg/iset.html>.

^{*} Can be downloaded from http://www.usc.edu/dept/civil_eng/Earthquake_eng/.

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